

Statement of Proposal:

VERIZON WIRELESS WILL BE REMOVING AND REPLACING THE EXISTING STREET LIGHT POLE LOCATED ON THE NORTHWEST CORNER OF THE INTERSECTION OF S 6TH AVE AND E 13TH ST. VERIZON WILL BE INSTALLING 3 (5G) ANTENNAS, A GROUND MOUNTED METER PEDESTAL AND ASSOCIATED FIBER VAULTS.

1. THIS WIRELESS TELECOMMUNICATIONS FACILITY WILL MEET THE HEALTH AND SAFETY STANDARDS FOR ELECTROMAGNETIC FIELD EMISSIONS AS ESTABLISHED BY THE FEDERAL COMMUNICATIONS COMMISSION OR ANY SUCCESSOR THEREOF, AND ANY OTHER FEDERAL OR STATE AGENCY.

2. THIS WIRELESS TELECOMMUNICATIONS FACILITY WILL MEET THE REGULATIONS OF THE FEDERAL COMMUNICATIONS COMMISSION REGARDING PHYSICAL AND ELECTROMAGNETIC INTERFERENCE.

3. LIGHTING OR SIGNS WILL BE PROVIDED ONLY AS REQUIRED BY FEDERAL OR STATE AGENCIES.

4. DEVELOPMENT AND CONSTRUCTION OF THIS PROJECT WILL COMPLY WITH ALL APPLICABLE CODES AND ORDINANCES.

5. EXISTING PARKING IS NOT AFFECTED BY THIS PROJECT.

Project Development Zone- Armory Park Neighborhood

North of Orange line: Code: HO-3 , South of Orange line: Code: HC-3



Description and photographs of type, color and texture of proposed materials
(can also be found in site plans)

Type:

GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE GRADE.

EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL.

METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING.

Coloring and texture will match current streetlight pole



AZ_TUC_BAILEY_5

Candidate A

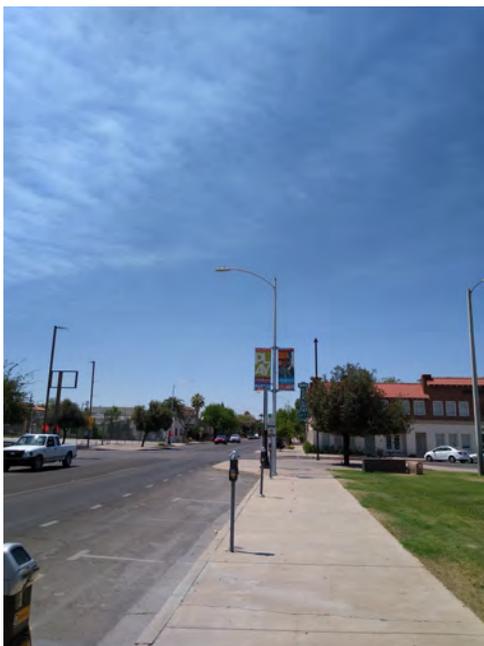
Site Name	Latitude	Longitude	Pole Number	Pole Location
A	32.218777	-110.968699	8412	2 LT N/O E 13TH ST W/S/O S 6TH AVE

Aerial

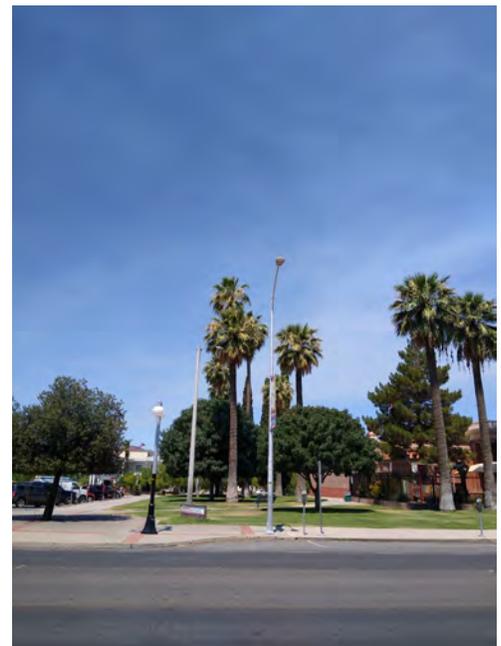


Street View

View from North



View from East



View from South



View to North



View to East



View to South



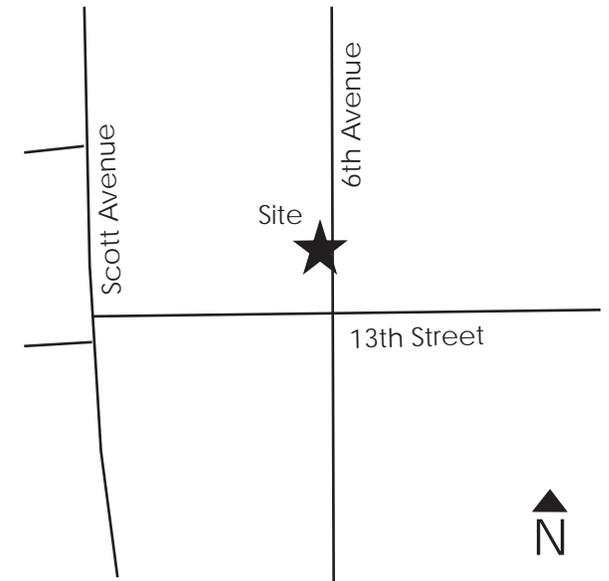
View to West



PHOTO SIMULATIONS

AZ_TUC_BAILEY_5

2LT N/O East 13th Street on W/S/O South 6th Avenue
Tucson, Arizona 85701



Note: Simulations are an artistic illustration created to represent how the proposed project may look once constructed. Simulations are create to match the current design as accurately as possible, but are not guaranteed to match the final build.



PHOTO SIMULATIONS

AZ_TUC_BAILEY_5



VIEW 1
Looking West

Before:



After:

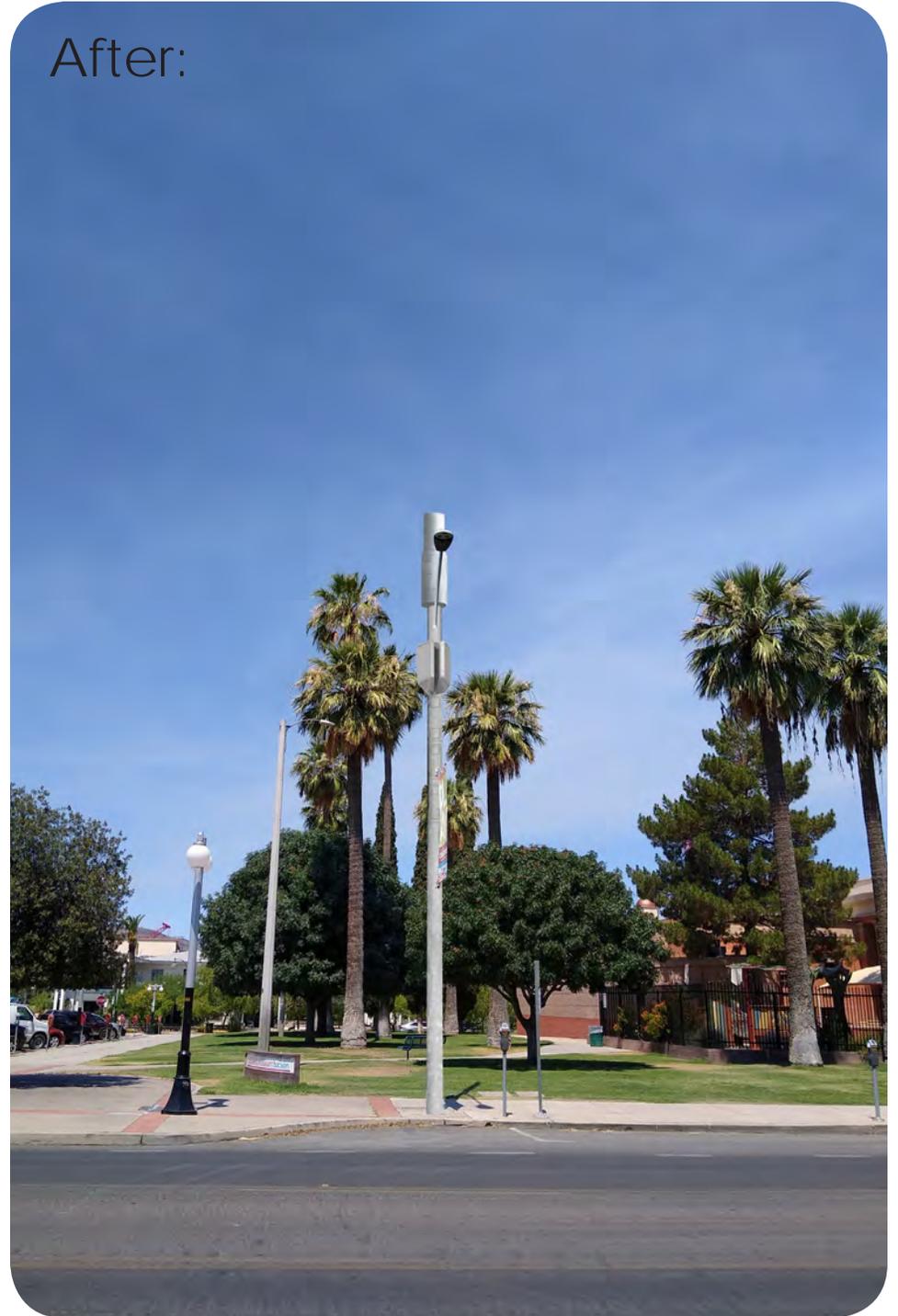


PHOTO SIMULATIONS

AZ_TUC_BAILEY_5

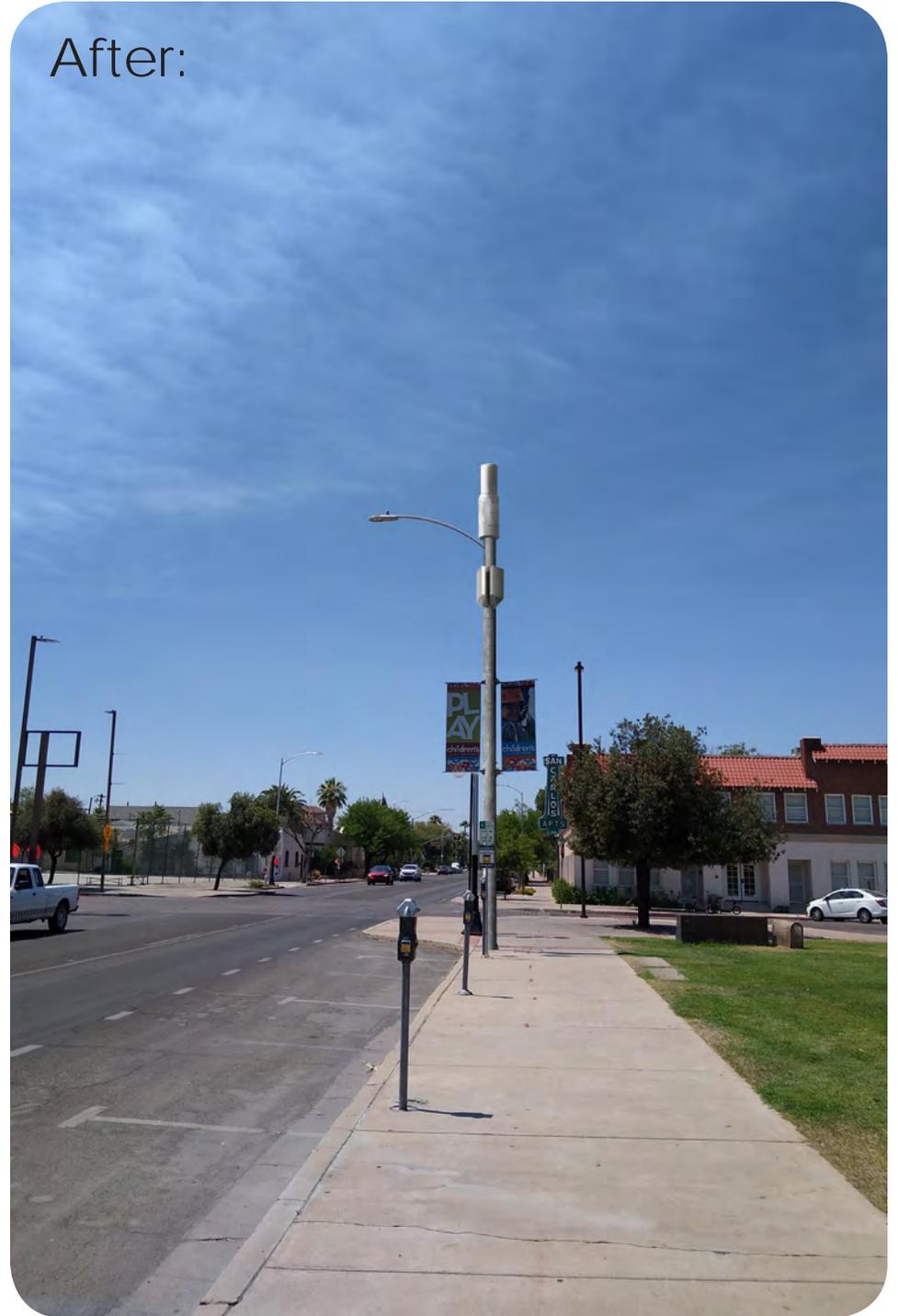


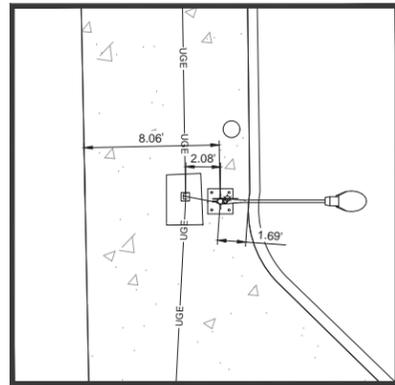
VIEW 2
Looking South

Before:

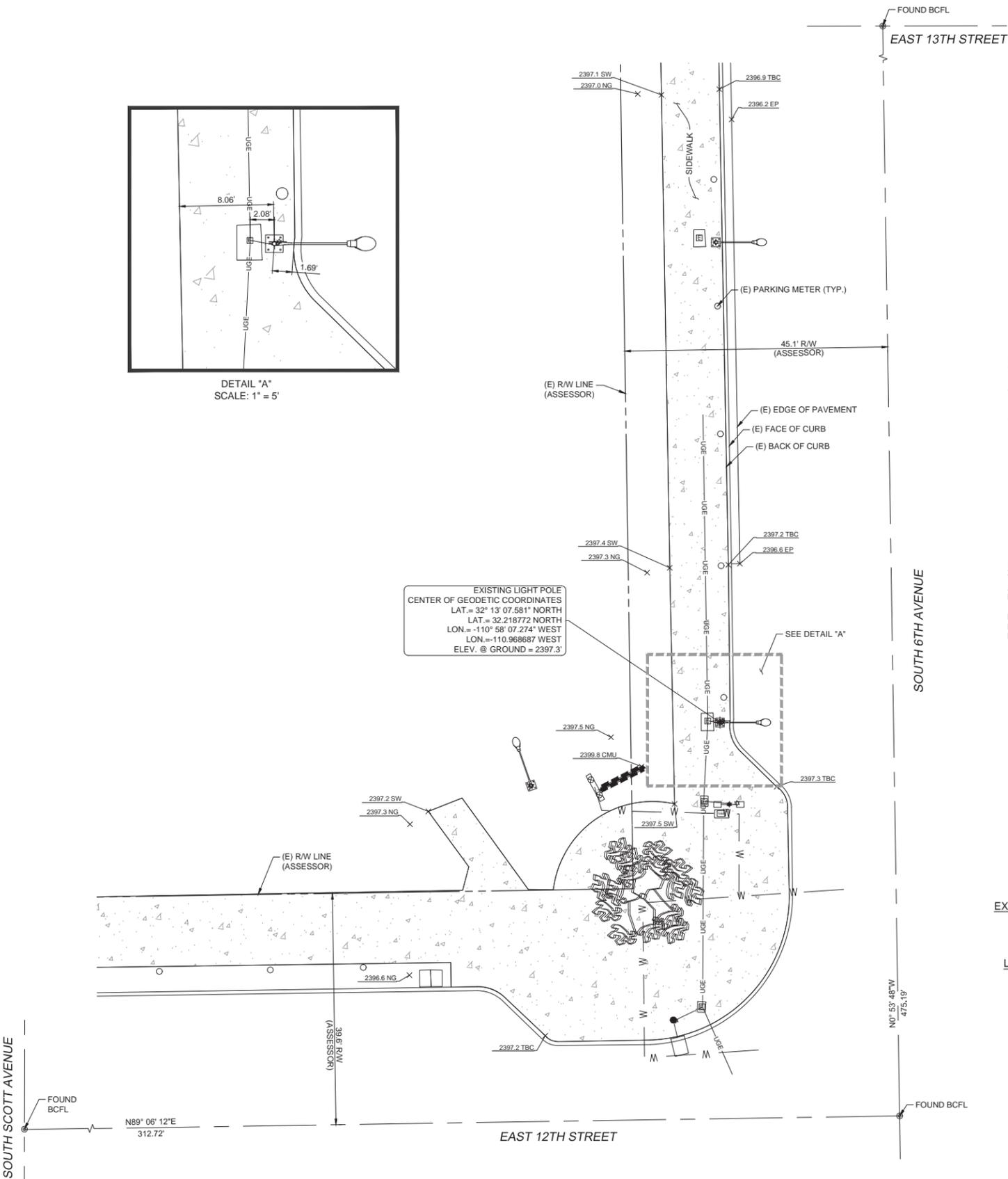


After:





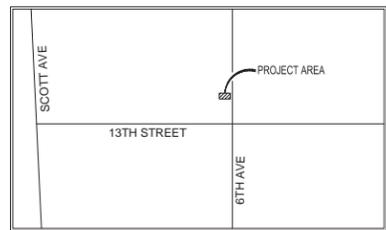
DETAIL "A"
SCALE: 1" = 5'



EXISTING LIGHT POLE
CENTER OF GEODETIC COORDINATES
LAT. = 32° 13' 07.581" NORTH
LON. = -110° 58' 07.274" WEST
ELEV. @ GROUND = 2397.3'

LEGEND

- ⊙ BRASS CAP FLUSH (BCFL)
- ELECTRICAL PULL BOX
- LIGHT POST
- STREET LIGHT W/ MAST ARM
- ⊞ WATER METER
- ⊞ BACKFLOW PREVENTER
- ⊞ DECIDUOUS TREE
- RIGHT-OF-WAY LINE
- CENTERLINE
- U/G ELECTRIC LINE
- WATER LINE



SURVEYOR NOTES

- A TITLE REPORT WAS NOT PROVIDED AT THE TIME OF THE SURVEY.
- SURVEYOR HAS NOT PERFORMED A SEARCH OF PUBLIC RECORDS TO DETERMINE ANY DEFECT IN TITLE.
- THE BOUNDARY SHOWN HEREON IS PLOTTED FROM RECORD INFORMATION AND DOES NOT CONSTITUTE A BOUNDARY SURVEY OF THE PROPERTY.
- SURVEYOR DOES NOT GUARANTEE THAT ALL UTILITIES ARE SHOWN OR THEIR LOCATIONS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR AND DEVELOPER TO CONTACT BLUE STAKE AND ANY OTHER INVOLVED AGENCIES TO LOCATE ALL UTILITIES PRIOR TO CONSTRUCTION. REMOVAL, RELOCATION AND/ OR REPLACEMENT IS THE RESPONSIBILITY OF THE CONTRACTOR.

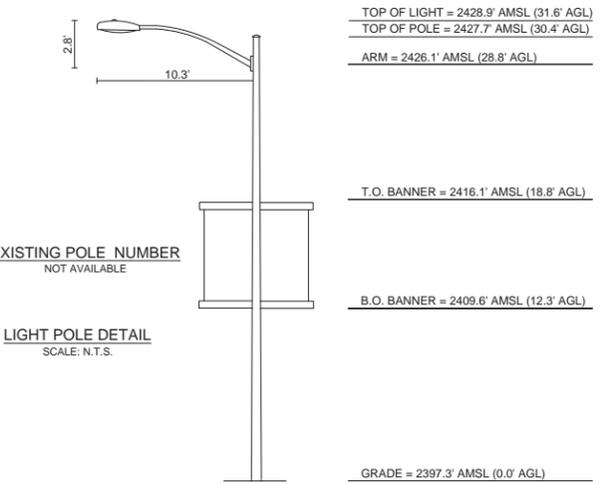
PROJECT META DATA

- ELEVATIONS SHOWN HEREON ARE REPRESENTED IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) ESTABLISHED FROM GPS DERIVED ELLIPSOID HEIGHTS, APPLYING GEOID 12B SEPARATIONS CONSTRAINING TO NGS CORS STATIONS PROVIDED IN THE "ONLINE POSITIONING USER SERVICE" (OPUS) SOLUTION FOR THIS SPECIFIC SITE.
- BEARINGS SHOWN HEREON ARE BASED UPON U.S. STATE PLANE NAD83 COORDINATE SYSTEM ARIZONA STATE PLANE COORDINATE ZONE CENTRAL, DETERMINED BY GPS OBSERVATIONS.
- FIELD WORK FOR THIS PROJECT WAS PERFORMED ON 03/31/20.

FLOOD ZONE DESIGNATION
THE PROPOSED LEASE PREMISES SHOWN HEREON APPEAR TO BE WITHIN FLOOD ZONE "X" AS DELINEATED ON THE FEDERAL EMERGENCY MANAGEMENT AGENCY FIRM MAP NO. 04019C2277L DATED 06/16/11.

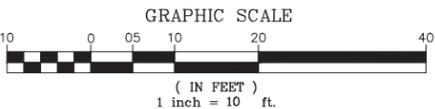
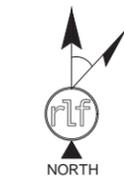
FLOOD ZONE "X" IS DEFINED AS: AREAS DETERMINED TO BE OUTSIDE 500-YEAR FLOODPLAIN; DETERMINED TO BE OUTSIDE THE 1% AND 0.2% ANNUAL CHANCE FLOODPLAINS.

LESSOR'S LEGAL DESCRIPTION
CITY OF TUCSON RIGHT-OF-WAY, ADJACENT TO PARCEL NO. 117170580



EXISTING POLE NUMBER
NOT AVAILABLE

LIGHT POLE DETAIL
SCALE: N.T.S.



WWW.STATE48CONSULTING.COM
8687 E. VIA DE VENTURA #115
SCOTTSDALE, AZ 85258
480-242-2477 602-821-3567

FIELD BY:	JDT
DRAWN BY:	GAC
CHECKED BY:	RLF

REVISIONS		
NO.	DATE	DESCRIPTION
0	04/02/20	PRELIMINARY



REUSE OF DOCUMENT
THE IDEAS & DESIGN INCORPORATED HEREON, AS AN INSTRUMENT OF PROFESSIONAL SERVICE, IS THE PROPERTY OF RLF CONSULTING, LLC & IS NOT TO BE USED FOR ANY OTHER PROJECT WITHOUT WRITTEN AUTHORIZATION OF RLF CONSULTING, LLC.

PROJECT No.
18001958
SITE NAME:
AZ_TUC_BAILEY_5
SITE ADDRESS:
(PIMA COUNTY) ADJACENT TO
PARCEL NO. 117170580

SHEET TITLE:
TOPOGRAPHIC SURVEY

SHEET NO.
LS-1
REVISION:

GENERAL STRUCTURAL NOTES

BUILDING CODE

2018 EDITION OF THE INTERNATIONAL BUILDING CODE

LOADS

WIND
WIND SPEED (ULTIMATE 3-SEC GUST), $V_{ult} = 105$ MPH
WIND EXPOSURE CATEGORY = C
RISK CATEGORY = II

SEISMIC
 $S_{ps} = 0.300g$ (MAX)
 $S_{D1} = 0.150g$ (MAX)
SITE SOIL CLASS = D
SEISMIC DESIGN CATEGORY = C
SEISMIC FORCE RESISTING SYSTEM = STEEL TELECOMMUNICATIONS POLE ($R = 1.5$)

THESE DRAWINGS ARE ISSUED FOR MULTI-USE WITHIN TUCSON, ARIZONA. THE LOAD CRITERIA LISTED ABOVE IS EXPECTED TO BE SUFFICIENT FOR TYPICAL CONDITIONS ON FLAT OPEN TERRAIN BUT DOES NOT TAKE INTO ACCOUNT LOCATIONS ON HILLS, CLIFFS, SUDDEN CHANGES IN TOPOGRAPHY AND/OR OTHER SPECIAL CONDITIONS. CONTACT EOR WITH QUESTIONS OR SPECIAL CONDITIONS.

FOUNDATIONS

DRILLED PIER FOUNDATIONS ARE BASED ON THE PRESUMPTIVE SOIL BEARING VALUES PROVIDED IN TABLE 1806.2, SOIL CLASS 5 AND HAVE BEEN INCREASED BY A FACTOR OF TWO PER 1806.3.4.
ALLOWABLE LATERAL BEARING PRESSURE = $100 \text{ PSF/FT} \times 2 = 200 \text{ PSF/FT}$.

CONCRETE

ALL CONCRETE WORK SHALL COMPLY WITH THE LATEST EDITION OF THE ACI. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED. PLACEMENT OF PLUMBING, CONDUITS, OR OTHER MATERIALS WITHIN CONCRETE FOUNDATIONS OR STRUCTURAL ELEMENTS IS PROHIBITED EXCEPT WHERE SHOWN.

SPECIFIED MINIMUM 28 DAY STRENGTH AS FOLLOWS:

DRILLED PIER CONCRETE: $f'_c = 3,000 \text{ PSI MIN}$

REINFORCING STEEL (REBAR)

ALL REINFORCING SHALL COMPLY WITH ACI AND CRSI SPECIFICATIONS. FOR #5 BARS AND LARGER USE ASTM A615 GRADE 60 DEFORMED BARS ($F_y = 60 \text{ KSI}$). FOR #4 BARS AND SMALLER USE ASTM A615 GRADE 40 DEFORMED BARS ($F_y = 40 \text{ KSI}$). ASTM A615 BARS ARE NOT TO BE WELDED. NO WELDING OF REINFORCING BARS IS PERMITTED FOR THIS PROJECT.

CLEAR DISTANCE FROM THE EDGE OF REINFORCING BAR TO THE EDGE OF CONCRETE SHALL BE PER ACI 318 AND IS AS FOLLOWS:

CONCRETE AGAINST EARTH = 3" CLR
CONCRETE AGAINST AIR = 1-1/2" CLR (FOR NO. 5 & SMALLER)

REINFORCING STEEL SHALL BE PLACED AS SHOWN IN THE PLANS AND MUST NOT BE MORE OR LESS THAN 3/8" OF THE DIMENSIONS SPECIFIED. THIS INCLUDES MINIMUMS AND CLEAR DISTANCES. ENSURE REINFORCING IS KEPT DRY AND IS PROPERLY SUPPORTED WITH CLEAR DISTANCES FROM SOILS.

ANCHOR RODS (ANCHOR BOLTS)

ANCHORAGE TO THE CONCRETE FOUNDATION IS ACHIEVED VIA A DOUBLE-NUT MOMENT JOINT. ANCHOR RODS SHALL BE TENSIONED TO THE SPECIFICATIONS BELOW. ANCHOR RODS SHALL BE THREADED AND NUTTED. CONTRACTOR SHALL ENSURE NUTS DO NOT SPIN OFF DURING VIBRATION OF CONCRETE BY PROVIDING AN ACCEPTABLE LOCKING MECHANISM OR BY TACK WELDING THE NUT TO ANCHOR ROD. NUTS AND ANCHOR RODS ARE TO BE GALVANIZED IN SAME PROCESS TO ENSURE WORKABLE THREADS.

ANCHOR BOLT GRADE: F1554 Gr 55

TIGHTENING

ANCHOR RODS SHALL BE LUBRICATED BEFORE TIGHTENING. TIGHTENING SHALL BE PERFORMED IN A STAR PATTERN. TOP NUTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION THEN LEVELING NUTS SHALL BE MADE SNUG TIGHT. CONFIRM TORQUE AT TOP NUT IS WITHIN THE INITIAL TORQUE SHOWN IN TABLE BELOW. MARK BOLTS AFTER INITIAL TORQUE IS ACHIEVED.

TOP NUTS SHALL THEN BE TENSIONED USING THE TURN OF THE NUT METHOD BY ROTATING THE NUT A TOTAL OF 1/3 TURN PAST INITIAL TORQUE. IT IS RECOMMENDED THAT THE TOTAL 1/3 TURN BE COMPLETED USING A MINIMUM OF (2) INCREMENTAL STEPS WITH STAR PATTERN TIGHTENING. USING A CALIBRATED TORQUE WRENCH, VERIFY THAT THE VERIFICATION TORQUE HAS BEEN REACHED.

AFTER AT LEAST 48 HOURS, THE CONTRACTOR SHALL RE-VISIT THE SITE AND CONFIRM THAT A TORQUE OF AT LEAST 110% OF THE VERIFICATION TORQUE CAN BE REACHED TO ENSURE BOLTS WILL REMAIN TENSIONED AND HAVE NOT RELAXED. INSTALL SECOND NUT OR JAMB NUT ON TOP OF ASSEMBLY AND TIGHTEN JAMB NUT TO BE SNUG TIGHT.

DO NOT OVER TIGHTEN. CONTACT EOR WITH ANY EXCESSIVE TIGHTENING, STRIPPED THREADS, OR OTHER CONCERNS.

TORQUE VALUES (FT-LBS)			
FOR: 1-1/4" DIA. F1554 Gr 55	INITIAL TORQUE	VERIFICATION TORQUE ($T_v = 0.12d_s T_m$)	110% x T_v (48 HOURS LATER)
	110 - 165	550	600

STRUCTURAL STEEL

ALL STRUCTURAL STEEL CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITION OF THE AISC STEEL CONSTRUCTION MANUAL AND ASTM STANDARDS. ALL STRUCTURAL STEEL MATERIAL MUST BE MILL CERTIFIED. ALL STRUCTURAL MEMBERS ARE TO BE HOT DIPPED GALVANIZED ACCORDING TO THE APPROPRIATE ASTM STANDARD. THE FOLLOWING STEEL GRADES SHALL APPLY UNLESS NOTED OTHERWISE.

ROUND HSS (POLE STEEL): ASTM A500 ($F_y = 42 \text{ KSI MIN}$)
LUMINAIRE: ASTM A500 ($F_y = 42 \text{ KSI MIN}$)
PLATE STEEL: ASTM A36 ($F_y = 36 \text{ KSI}$)
MISC STEEL: ASTM A36 ($F_y = 36 \text{ KSI}$)

WELDING:

ALL WELDING SHALL COMPLY WITH THE LATEST EDITION OF THE AWS STANDARD. ALL WELDING SHALL UTILIZE TYPE E70 RODS. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS. THESE PLANS DO NOT INDICATE WHETHER WELDING MUST BE DONE IN SHOP OR FIELD. CONTRACTOR CAN PROVIDE SHOP OR FIELD WELDING AT CONTRACTORS DISCRETION AS BEST SUITS THE PROJECT'S MEANS AND METHODS.

BOLTS:

ALL THRU-BOLT TYPE CONDITIONS SHALL UTILIZE A WASHER AT EACH SIDE OF THE CONNECTION AND TIGHTENED TO A SNUG TIGHT CONDITION. SEE DETAILS FOR BOLT SIZE AND GRADE.

GENERAL NOTES

ALL WORK PRESENTED WITHIN THESE DRAWINGS AND DETAILS SHALL ONLY BE PERFORMED BY A CONTRACTOR THAT IS EXPERIENCED AND KNOWLEDGEABLE IN THE TYPE OF WORK BEING PERFORMED AND HAS A HISTORY OF COMPLETING SIMILAR PROJECTS. ONLY A CONTRACTOR THAT IS LICENSED AND REGISTERED IN THE STATE WHERE THE WORK IS TO BE PERFORMED SHALL BE PERMITTED TO PERFORM THE WORK.

CONTRACTOR MUST CONFORM TO THE CITY STANDARDS, SPECIFICATIONS, & AMENDMENTS. SEE CITY SPECIFICATIONS AND DETAILS FOR ADDITIONAL INFORMATION & NOTIFY THE BUILDING OFFICIAL AND EOR OF ANY DISCREPANCIES. ADDITIONALLY, CONTRACTOR MUST BE FAMILIAR WITH THE ADOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.

THE STRUCTURAL PLANS AND DETAILS DEPICT THE REQUIREMENTS FOR THE FINISHED STRUCTURAL ELEMENTS. THESE PLANS DO NOT PROVIDE DIRECTION FOR ELECTRICAL, MECHANICAL, OR OTHER SCOPES. THE PLANS AND DETAILS DO NOT PROVIDE THE CONTRACTOR WITH "MEANS AND METHODS" OF CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL REQUIRED MEASUREMENTS AND INFORMATION IN ORDER TO MEET THE STRUCTURAL REQUIREMENTS OF THESE PLANS. ANY ADDITIONAL INFORMATION NEEDED FROM THE ENGINEER OF RECORD (EOR) CAN BE OBTAINED WITH A FORMAL REQUEST FOR INFORMATION (RFI).

THE PLANS AND DETAILS DO NOT PROVIDE ENGINEERING FOR ANY SHORING, TEMPORARY BRACING, SCAFFOLDING, OR OTHERWISE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE A SAFE WORK ENVIRONMENT AND TO OBTAIN ANY ADDITIONAL ENGINEERING SERVICES THAT ARE NEEDED IN ORDER TO SUPPORT TEMPORARY LOADS OR LOADS DUE TO CONSTRUCTION ACTIVITIES. THE ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE SEQUENCING, PROCEDURES, OR TECHNIQUES USED BY THE CONTRACTOR.

SPECIAL STRUCTURAL INSPECTIONS

THE SPECIAL INSPECTIONS LISTED BELOW ARE REQUIRED PER CH. 17 OF THE INTERNATIONAL BUILDING CODE.

DRILLED PIER CONSTRUCTION

1. CONTINUOUS INSPECTION OF DRILLING OPERATIONS.
2. VERIFICATION OF SOIL STRATA CONFORMANCE TO PRESUMPTIVE SOIL CLASS.
3. VERIFICATION OF DRILLED SHAFT SIZE AND CONFORMANCE TO FOUNDATION DETAIL.

CONCRETE CONSTRUCTION

1. NO CONCRETE INSPECTION & TESTING OF SPECIMENS IS REQUIRED FOR PLACEMENT OF PIER FOUNDATION CONCRETE. FOUNDATION DESIGN IS BASED ON $f'_c = 2,500 \text{ PSI}$. ($f'_c = 3,000 \text{ PSI}$ CONCRETE TO BE PROVIDED PER CONCRETE SECTION OF GSN).

STEEL REINFORCING

1. IN-PLACE REINFORCING IN FOUNDATIONS PRIOR TO CONCRETE PLACEMENT.
2. VERIFICATION OF CONFORMANCE TO SPECIFICATIONS AND DETAILS.

ANCHOR BOLTS

1. VERIFICATION OF PROPER MATERIAL SPECIFICATIONS AND CONFORMANCE TO DETAILS.
2. VERIFICATION OF PROPER LUBRICATING AND TIGHTENING OF BOLTS.

WELDING

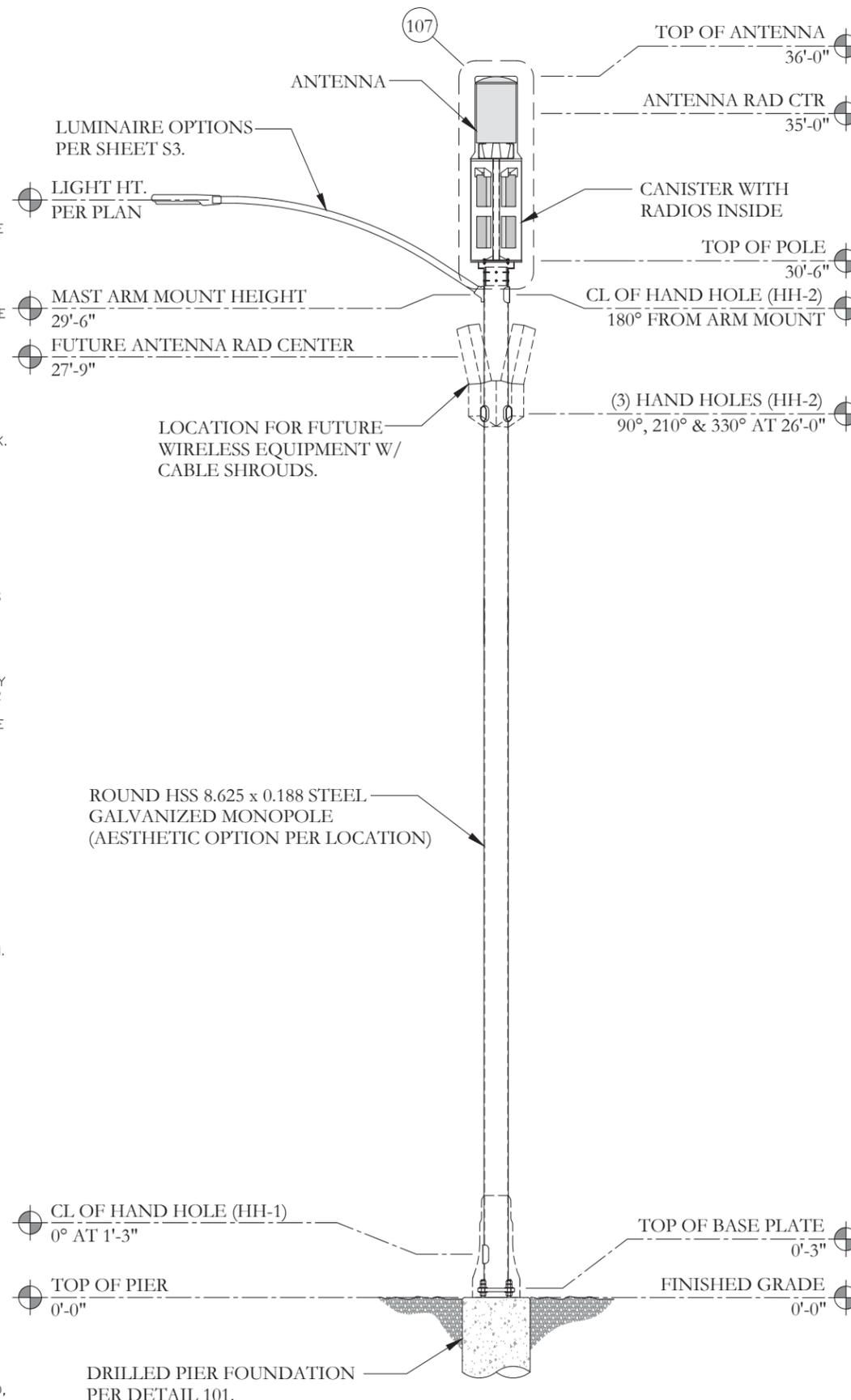
1. PERIODIC INSPECTION OF ALL FIELD WELDS.
2. CONTINUOUS INSPECTION AS REQUIRED BELOW:
 - a. NO PJP, CJP, OR MULTI-PASS FILLET WELDS ARE SPECIFIED FOR THIS PROJECT.

RESPONSIBILITIES OF THE CONTRACTOR

1. ANY DEVIATIONS MUST BE APPROVED IN WRITING FROM THE EOR AND MUST BE ORIGINATED IN WRITING BY THE CONTRACTOR WITH A REQUEST FOR INFORMATION.
2. WHERE THE WORK IS REQUIRED TO BE COMPLETED IN THE PRESENCE OF THE SPECIAL INSPECTOR, THE CONTRACTOR SHALL BE SURE TO PERFORM THE WORK UNDER THE OBSERVANCE OF THE SPECIAL INSPECTOR.
3. AREAS TO BE INSPECTED BY THE SPECIAL INSPECTOR ARE TO BE MADE SAFELY ACCESSIBLE FOR INSPECTION.
4. FOR ANY QUESTIONS REGARDING SPECIAL INSPECTIONS, CONTACT THE EOR.

RESPONSIBILITIES OF THE SPECIAL INSPECTOR

1. THE SPECIAL INSPECTOR SHALL VISIT THE SITE AND ENSURE THE WORK PERFORMED CONFORMS TO THE DETAILS AND SPECIFICATIONS SHOWN ON THE PLANS.
2. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE OR SUGGEST ANY DEVIATIONS FROM WHAT IS SHOWN ON THE PLANS.
3. THE SPECIAL INSPECTOR MUST BE KNOWLEDGEABLE IN THE WORK BEING PERFORMED, KNOW THE MANUFACTURER REQUIREMENTS AND UNDERSTAND ITEMS REQUIRING INSPECTION AND OBSERVATION.
4. THE SPECIAL INSPECTOR MUST PROVIDE WRITTEN INSPECTION REPORTS TO BOTH THE ENGINEER OF RECORD AND THE BUILDING OFFICIAL.
5. ANY DISCREPANCIES REQUIRING CORRECTION MUST BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF LEFT UNCORRECTED, THE DISCREPANCIES MUST BE MADE KNOWN TO THE EOR AND BUILDING OFFICIAL.



**A POLE ELEVATION
NTS**



VZTUTYPXXII
SMALL CELL LIGHT POLE
MULTI USE DESIGN
TUCSON, AZ

REV	ISSUED	DATE
0	ISSUED FOR PERMIT	03.02.20



CES
CaliberEngineering
Solutions
INTEGRITY. QUALITY. EXPERTISE.
2487 S. GILBERT RD. STE. 106-(607)
GILBERT, AZ 85295
480.329.0493
WWW.CALIBER-ES.COM

JOB: 20-S024 | ENG: MEN

GSN & ELEVATION

S1 | 0

VZTUTYPXXII
SMALL CELL LIGHT POLE
MULTI USE DESIGN
TUCSON, AZ

REV	ISSUED	DATE
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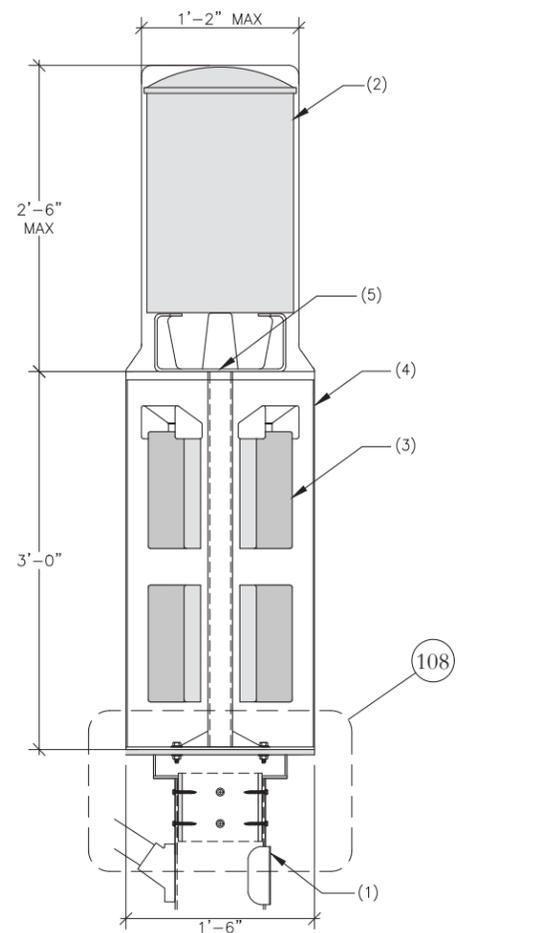


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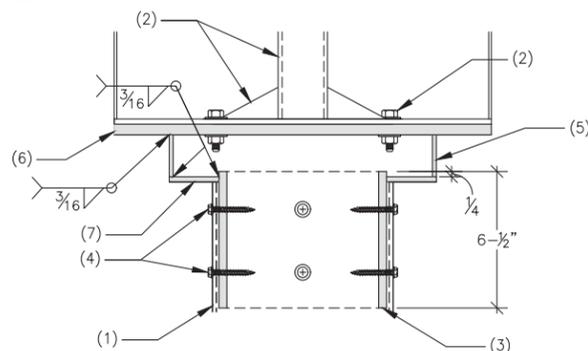
POLE DETAILS

S2 0



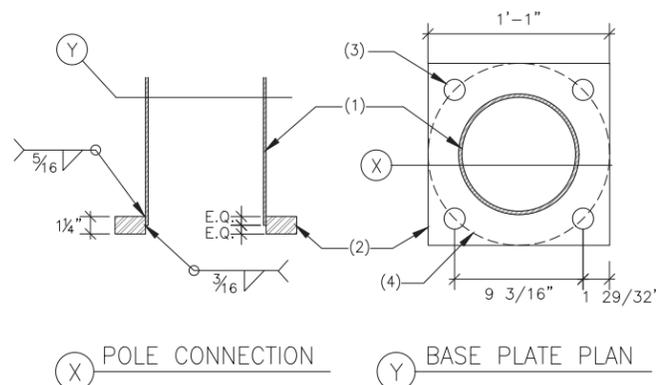
1. STEEL MONOPOLE PER ELEVATION.
2. ANTENNA.
3. RADIO EQUIPMENT.
4. EQUIPMENT CANISTER & SHROUD BY COMMSCOPE.
5. ANTENNA BRACKET & ATTACHMENT PROVIDED BY ANTENNA MANUFACTURER.

107 TOP OF POLE ASSEMBLY
NTS



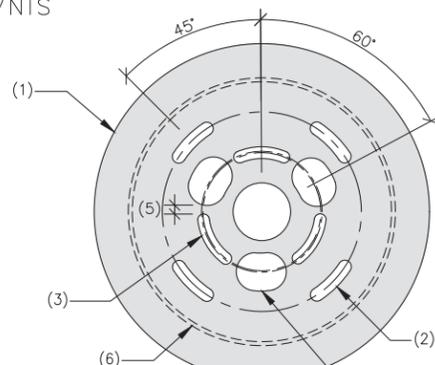
1. STEEL MONOPOLE PER ELEVATION.
2. EQUIPMENT CANISTER FRAME & ATTACHMENT TO MATE PLATE PROVIDED BY COMMSCOPE.
3. ROUND HSS 8x3/8 x 6-1/2" LONG SLIP FITTING.
4. (2) SETS OF (4) 1/4" DIA. TEK SCREWS. (2) SCREWS AT EACH SIDE OF SQUARE TUBE AT 3-1/2" O.C. VERTICALLY (8 TOTAL).
5. 12-3/4" OD x 3/16" THICK x 2" TALL ROUND TUBE / RING.
6. MATE PLATE PER DETAIL 105.
7. MONOPOLE TOP PLATE PER DETAIL 106.

108 TOP ASSEMBLY ATTACHMENT TO POLE
NTS



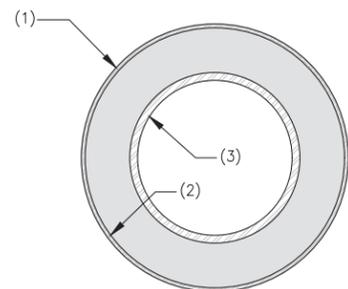
1. STEEL MONOPOLE BASE.
2. 13x13x1-1/4 STEEL BASE PLATE.
3. 1-1/2" DIA. HOLES FOR ANCHOR BOLTS.
4. 13" DIA. BOLT CIRCLE.

104 BASE PLATE & BASE PLATE CONNECTION
NTS



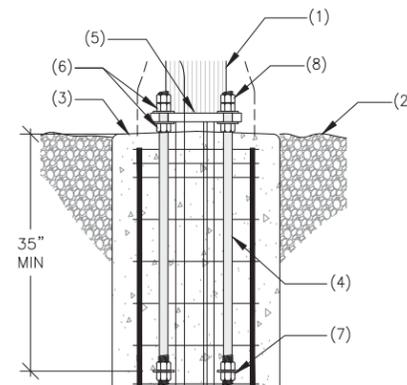
1. 18" DIA. x 1/2" THICK STEEL PLATE W/ 2-3/4" DIA. HOLE AT CENTER.
2. (4) 5/8" WIDE x 2-1/2" LONG SLOTTED HOLES ON A 9-1/2" DIA. BOLT CIRCLE - EQUALLY SPACED.
3. (3) 9/16" WIDE x 2-5/8" LONG SLOTTED HOLES ON A 5-5/8" DIA. BOLT CIRCLE - EQUALLY SPACED.
4. (3) 1-13/16" WIDE x 2-1/2" LONG SLOTTED HOLES ON A 5-3/4" DIA. BOLT CIRCLE - EQUALLY SPACED.
5. 3/8" MIN CLEAR BETWEEN HOLES.
6. 2" TALL RING BELOW PER DETAIL 108.

105 MATE PLATE
NTS



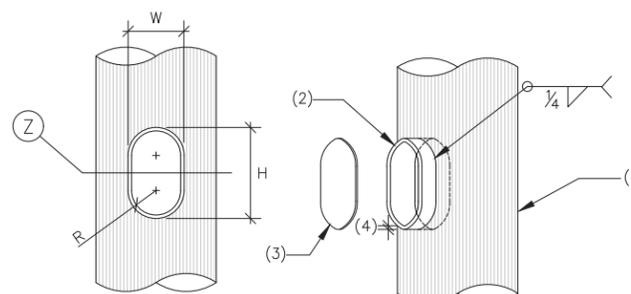
1. 12-3/4" DIA. x 1/4" THICK STEEL PLATE W/ 8-1/8" DIA. HOLE AT CENTER.
2. 2" TALL RING ABOVE PER DETAIL 108.
3. 8" OD ROUND HSS TUBE SLIP FITTING PER DETAIL 108.

106 MONOPOLE TOP PLATE
NTS

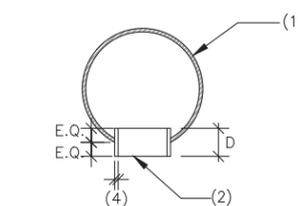


1. STEEL MONOPOLE PER ELEVATION.
2. FINISHED GRADE.
3. DRILLED PIER PER DETAIL 101.
4. (4) 1-1/4" DIA. x 44" LONG ANCHOR RODS. LEAVE 6" PROJECTION ABOVE PIER.
5. BASE PLATE AND CONNECTION TO POLE PER DETAIL 104.
6. HEAVY HEX NUT W/ STRUCTURAL WASHER AT EACH SIDE OF PLATE - PRETENSIONED - TIGHTEN PER GSN.
7. MIN 3x3x1/4 PLATE WASHER W/ HEAVY HEX NUT EACH SIDE. ENSURE NO SPIN OFF PER GSN.
8. AFTER TENSIONING ROD - INSTALL SECOND NUTS SNUG TIGHT.

102 BASE ANCHORAGE TO DRILLED PIER
NTS



X ELEVATION Y ISOMETRIC

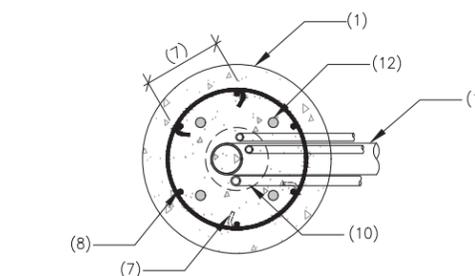
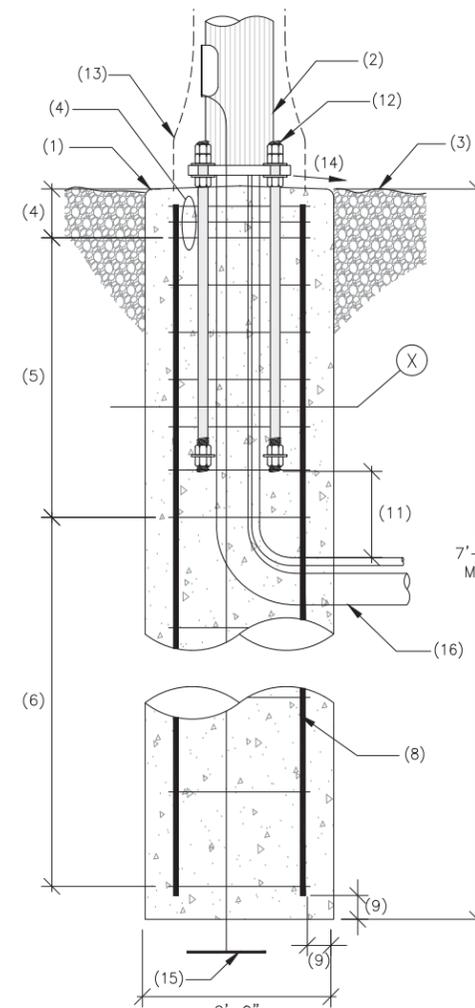


Z PLAN

1. STEEL MONOPOLE PER ELEVATION.
2. HAND HOLE - SEE POLE ELEVATION FOR LOCATION AND MARK. SEE SCHEDULE BELOW FOR CORRESPONDING SIZE.
3. PROVIDE COVER PLATE ASSEMBLY AT HAND HOLES.
4. THICKNESS PER 't' IN THE TABLE BELOW.

HAND HOLE SCHEDULE					
MARK	HEIGHT (H)	WIDTH (W)	DEPTH (D)	RADIUS (R)	THICK (t)
HH-1	6 1/2 IN	4 IN	2 IN	1 3/4 IN	1/4 IN
HH-2	5 1/2 IN	3 IN	2 IN	1 1/4 IN	1/4 IN

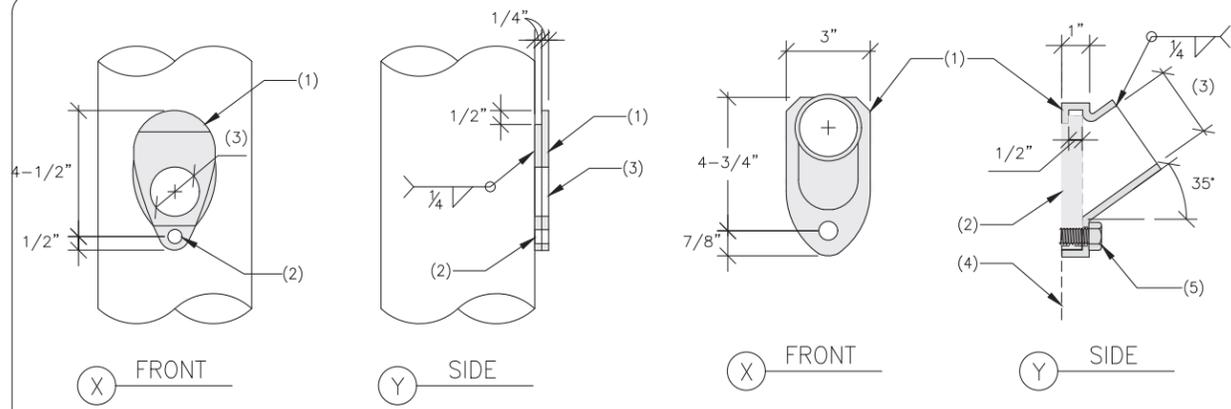
103 HAND HOLE SCHEDULE
NTS



X SECTION

1. CONCRETE DRILLED PIER.
2. STEEL MONOPOLE PER ELEVATION.
3. FINISHED GRADE.
4. (3) #3 TIES EQUALLY SPACED IN TOP 6" OF CONCRETE PIER.
5. #3 TIES AT 6" O.C. - CONTINUED PAST ANCHOR BOLT EMBEDMENT.
6. #3 TIES AT 12" O.C MAX FOR REMAINDER OF PIER DEPTH.
7. LAP TIES 12" MIN. STAGGER LAPS 180°.
8. (6) #6 LONGITUDINAL BARS EQUALLY SPACED.
9. MIN 3" CLR PER GSN.
10. CONTRACTOR TO BIND VERTICAL RUN OF CONDUIT TOGETHER SUCH THAT BOUND CONDUIT RUN DOES NOT EXCEED 8" DIA CICLE. PLACE VERTICAL RUN IN CENTER OF DRILLED PIER.
11. 12" MIN.
12. ANCHOR RODS AND ANCHORAGE PER DETAIL 102.
13. DECORATIVE CLAMHELL BASE COVER TBD BY THE CITY.
14. SLOPE CONCRETE DOWN & AWAY FROM CENTER.
15. GROUND PER CITY REQUIREMENTS.
16. CONDUIT - INSTALL PER WIRELESS CARRIER & CITY STANDARDS.

101 DRILLED PIER FOUNDATION
NTS

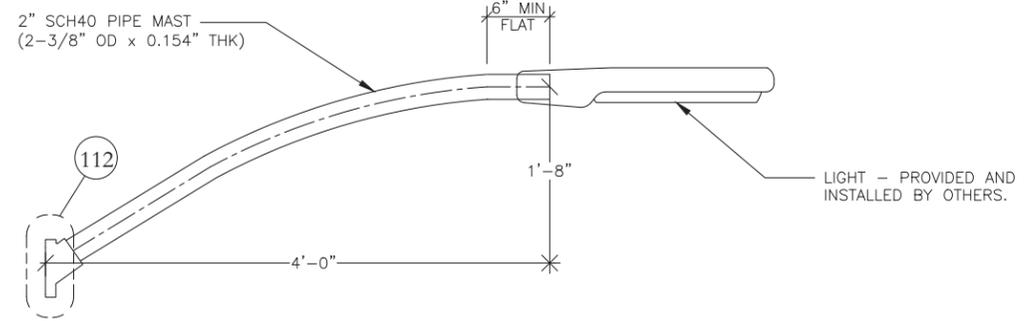


1. APS SIMPLEX FITTING SHOE.
2. 1/2" DIA. HOLE. DRILL AND TAP TO RECIEVE 1/2" HEX HEAD CAP SCREW.
3. 1-3/4" DIA. HOLE.

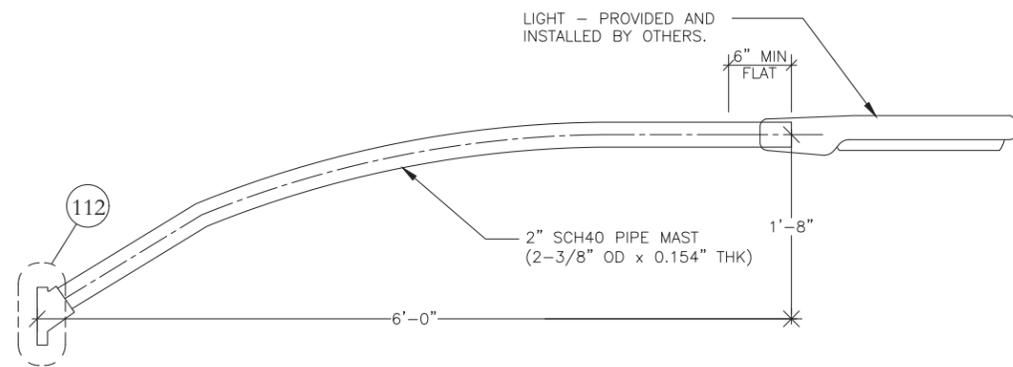
1. APS SIMPLEX FITTING FOOT (MIN 1/4" THICK STEEL).
2. APS SIMPLEX FITTING SHOE PER DETAIL 113.
3. OPENING TO RECIEVE A 2" NOMINAL (2-3/8" OD) PIPE.
4. LINE OF FACE OF POLE.
5. 1/2" DIA. x 5/8" HEX HEAD CAP SCREW (MIN A307 OR GR 5).

113 FITTING SHOE
NTS

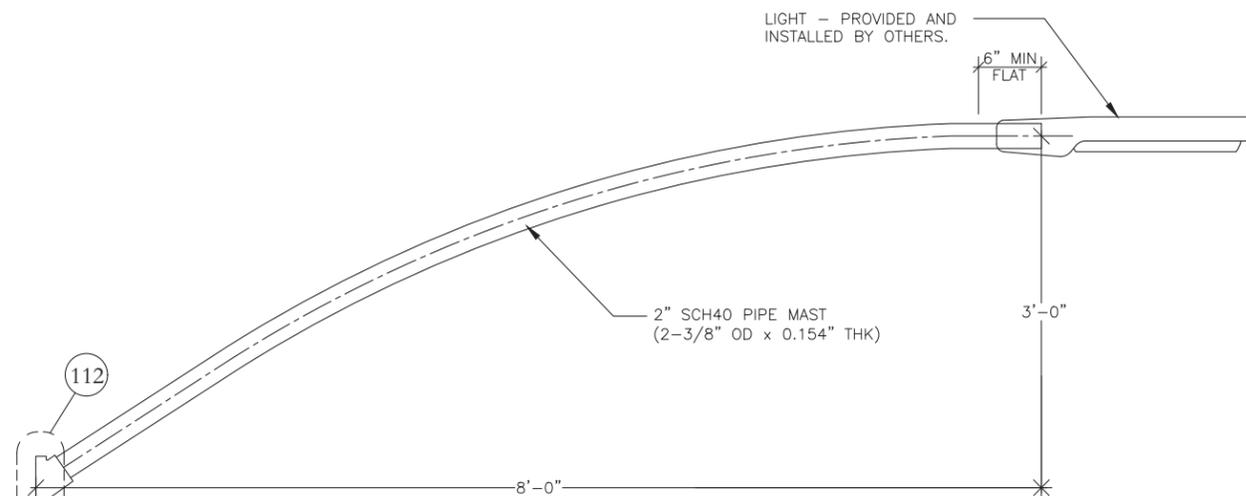
112 FITTING FOOT
NTS



109 4'-0" x 1'-8" LUMINAIRE MAST ARM
NTS



110 6'-0" x 1'-8" LUMINAIRE MAST ARM
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111 8'-0" x 3'-0" LUMINAIRE MAST ARM
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VZTUTYPXXII
SMALL CELL LIGHT POLE
MULTI USE DESIGN
TUCSON, AZ

REV	ISSUED	DATE
0	ISSUED FOR PERMIT	03.02.20



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LIGHT MAST DETAILS

S3 | 0